

City of Warrenton
South Water Reservoir
RFP

ADDENDUM NO. 2 – January 25, 2010

Addendum No. 2 hereby amends the contract documents and drawings for the 2010 *South Water Reservoir*. It is essential that prospective bidders note the contents of this addendum and that the *City of Warrenton* be made aware that the addendum has been received. Therefore, acknowledge receipt by inserting the number of this addendum in the space provided on your Proposal in the bid documents.

Bidding Information and Forms

Contract documents attached are as follows:

- Contract
- Bid Quantity (Omit item 410 Stream Bypass System and item 825 – 6' Chain link Fence without Barbed Wire)
- Sheets C1.1, C1.2, C2.0, C2.1 and C2.2 (for contour elevation clarification only)

Clarifications

The only prequalification for bidding on this work is having a team member at the Mandatory Pre-Bid meeting. No other prequalification is required.

Special Inspection will be required and will be the responsibility of the contractor.

Geotechnical

On Page 11 of the Geotechnical Report, it indicates that the structural fill must extend 2-feet beyond the edges of the footing. This should be revised to 3-feet to allow for the perimeter drain as shown on Sheet C5.3 detail 6 Tank Foundation With Perimeter Drain.

The Geotechnical Engineer recommends an ultimate value of 0.45 for the coefficient of friction for mass concrete cast directly on siltstone or angular structural fill (crushed rock).

If additional lateral resistance is required, passive earth pressure against the perimeter footing can be computed on the basis of an equivalent fluid having a unit weight of 225 pcf for limiting lateral deflections of ¼ to ½ in. This passive earth pressure assumes the backfill for the footings and walls is placed as granular structural (compacted) fill and does not slope down away from the tank. Lateral deflections greater than ½ in. would be needed to develop a larger passive earth pressure value.

The maximum bearing Capacity is 5ksf.

The at rest Lateral Earth pressure will be 45 pcf.

The recommend using an inverse triangular distribution of 10H to account for seismic earth pressures, where 'H' is the depth of the embedded wall in feet, the resultant is in psf, and the resultant is applied at 0.6H from the base of the embedded reservoir wall. Surcharge loads at the ground surface can be evaluated using the criteria provided on Figure 4.

Technical Specifications

- Revise Division 2 – Site work
 02221 Excavating, Backfilling and Compacting for Structures – Paragraph 3.3 A.6 Stockpiling of material is limited to that material earmarked to be reused as construction fill or top soil and is the responsibility of the contractor to both locate within the work area and protect against erosion.
 02620 Ductile Iron Pipe Fittings and Special Items – No double thickness cement lining is required. Class 250 18 inch pipe and class 200 24 inch pipe shall be used.
- Revise Section 13010 & 13020– The Type I tank may provide a dome roof of equal design to a Type III.
 All reinforcing steel that passes through the vertical wall joints shall be galvanized or epoxy coated.
 A safety cage as shown on sheet C5.4 detail 5 Exterior Ladder with Security Enclosure is required.
 All floor concrete shall include a shrinkage reducing admixture.
 Single wire or strands within the Chlorine ion shotcrete are not required to be coated.
 A flexible wall to roof connection is not required unless assumed in your tank design.

Plans

- Sheet C5.3 Detail 6 ASTM C33 67 type crushed is an acceptable alternative for key note 1 that reads “Free draining crushed rock fill: ¾” to 1.5” gradation, less than 2% passing no. 200 sieve.